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DYNAMIC STRESSES IN AN ELASTIC  
CYLINDRICAL LINING OF ARBITRARY THICKNESS  
IN AN ELASTIC MEDIUM

TECHNICAL DOCUMENTARY REPORT NO. ESD-TR-65-112

APRIL 1965

C. C. Mow

W. L. McCabe

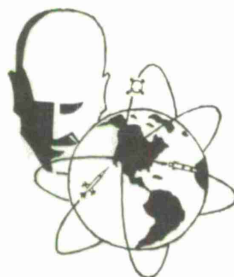
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Prepared for  
DIRECTOR OF ANALYSIS  
DEPUTY FOR ADVANCED PLANNING  
ELECTRONIC SYSTEMS DIVISION  
AIR FORCE SYSTEMS COMMAND  
UNITED STATES AIR FORCE

L. G. Hanscom Field, Bedford, Massachusetts



Project 607

Prepared by

THE MITRE CORPORATION  
Bedford, Massachusetts

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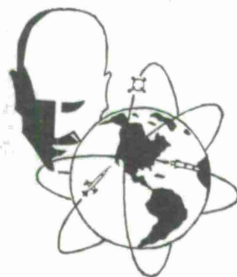
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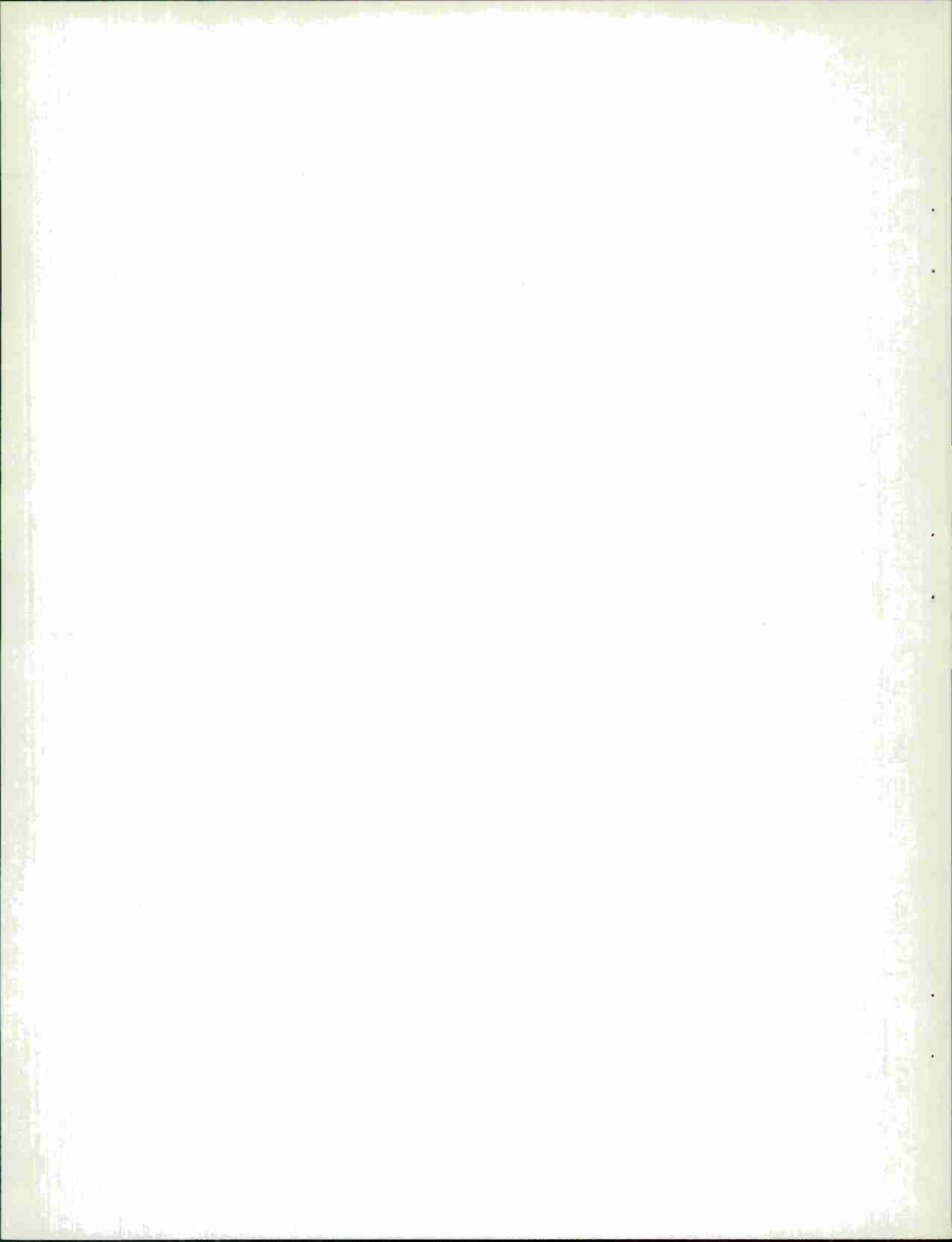
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## FOREWORD

The authors wish to thank Charles McCarthy of The MITRE Corporation for obtaining the numerical results, and Prof. Y.H. Pao of Cornell University for his advice during the course of the study.



DYNAMIC STRESSES IN AN ELASTIC  
CYLINDRICAL LINING OF ARBITRARY THICKNESS  
IN AN ELASTIC MEDIUM

ABSTRACT

Dynamic stresses in a thick-wall elastic cylinder in an infinite elastic medium during passage of plane, compressional waves are investigated. Dynamic stresses around the cylinder in the elastic medium are also determined. Numerical results for two different cylinders with ratios of outer radius to inner radius ranging from 1.05 to 1.20 are presented in a dimensionless form. It is shown that increasing thickness does not, in general, reduce stresses in the cylinder; in addition, dynamic stresses at certain wave numbers are higher than the corresponding static value.

REVIEW AND APPROVAL

This technical documentary report has been reviewed and is approved.

*Francis J. Dillon Jr.*

FRANCIS J. DILLON, JR.  
Col., USAF  
Director of Analysis  
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## SECTION I

### INTRODUCTION

The problem of dynamic stresses in an infinite medium containing cavities, rigid inclusions, and elastic inclusions has been studied extensively [1, 2, 3, 4] the problem of scattering or diffraction of sound or stress waves by thin elastic shells in fluid and elastic media has also been the subject of many investigations. [5, 6, 7]

The problem presented herein is the response of a thick-wall cylinder in an infinite elastic medium subjected to a progressing compressional wave. It is assumed that the thick-wall cylinder is of infinite extent and is embedded in and bound to an infinite elastic medium. A plane compressional wave of harmonic time variation propagates in the positive X-direction (Fig. 1) and impinges on the embedded cylinder.

It is known that if the material constants ( $\lambda$ ,  $\mu$ , and  $\rho$ ) of the cylinder and the elastic medium are different, scattering will occur at the interface of the cylinder and the surrounding medium. If, however, the material constants are the same, the problem should reduce to the simple cavity case presented in reference 1.

In general, at the boundary of the cylinder and the surrounding medium, the incident compressional waves are reflected and refracted as compressional and shear waves. Hence, there will be two refracted waves propagating into the cylinder. These waves will be reflected at the traction-free boundary of the inner surface of the cylinder. Therefore, seven waves exist; one incident

wave, two reflected waves in the elastic medium, and four in the cylinder due to refraction and reflection.

The solution of this problem involves finding the coefficients associated with the six unknown waves. This is accomplished by four equations of continuity at the interface and two boundary conditions at the traction-free inner surface of the cylinder.

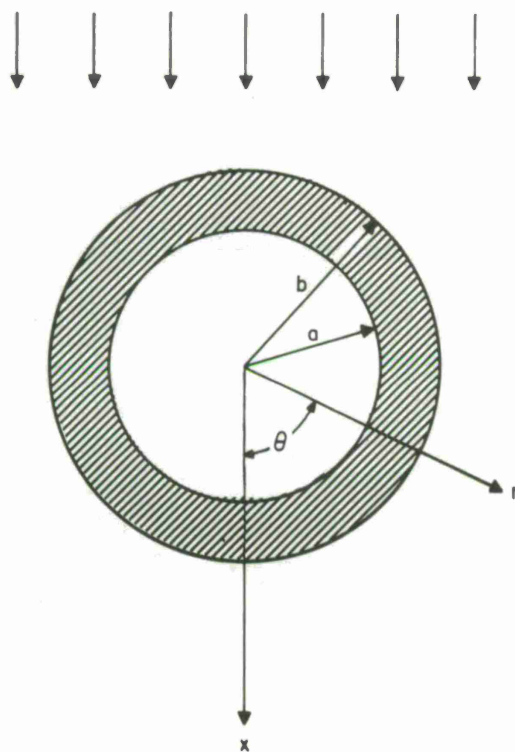


Fig. 1 Cylindrical Lining

## SECTION II

### GENERAL THEORY

If it is assumed that the cylinder is infinite in extent, the problem becomes one of generalized plane strain. The displacement equation of motion is

$$(\lambda + \mu) \nabla \nabla \cdot \underline{u} + \mu \nabla^2 \underline{u} = \rho \ddot{\underline{u}} \quad (1)$$

where

$\underline{u}$  is the displacement vector

$\nabla$  is the gradient operator

$\lambda$  and  $\mu$  are the Lamé Constants

$\rho$  is the density.

The displacement vector  $\underline{u}$  can be represented by a scalar potential and a vector potential; in the case of plane strain this is

$$\underline{u} = \nabla \phi + \nabla \times (\underline{e}_z \psi) \quad (2)$$

Each potential function then satisfies a scalar wave equation

$$c_\alpha^2 \nabla^2 \phi = \ddot{\phi} \quad (3)$$

$$c_\beta^2 \nabla^2 \psi = \ddot{\psi} \quad (4)$$

In Equations (2), (3), and (4),  $\underline{e}_z$  is a unit vector along the axis of the cylinder and

$$c_{\alpha}^2 = \frac{\lambda + 2\mu}{\rho} \quad c_{\beta}^2 = \frac{\mu}{\rho}$$

As shown in Fig. 1, in plane polar coordinates  $(r, \theta)$  the scalar form of Equation (2) is

$$\begin{aligned} u_r &= \frac{\partial \phi}{\partial r} + \frac{1}{r} \frac{\partial \psi}{\partial \theta} \\ u_{\theta} &= \frac{1}{r} \frac{\partial \phi}{\partial \theta} - \frac{\partial \psi}{\partial r} \end{aligned} \quad (5)$$

and the stresses are related to the potentials by

$$\begin{aligned} \tau_{rr} &= \lambda \nabla^2 \phi + 2\mu \left( \frac{\partial^2 \phi}{\partial r^2} + \frac{1}{r} \frac{\partial^2 \psi}{\partial r \partial \theta} - \frac{1}{r^2} \frac{\partial \psi}{\partial \theta} \right) \\ \tau_{\theta\theta} &= \lambda \nabla^2 \phi + 2\mu \left( \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2} + \frac{1}{r} \frac{\partial \phi}{\partial r} - \frac{1}{r} \frac{\partial^2 \psi}{\partial r \partial \theta} + \frac{1}{r^2} \frac{\partial \psi}{\partial \theta} \right) \\ \tau_{r\theta} &= 2\mu \left( \frac{1}{r} \frac{\partial^2 \phi}{\partial r \partial \theta} - \frac{1}{r^2} \frac{\partial \phi}{\partial \theta} + \frac{1}{r} \frac{\partial \psi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \psi}{\partial \theta^2} - \frac{1}{2} \nabla^2 \psi \right) \end{aligned} \quad (6)$$



### SECTION III

#### INCIDENT REFLECTED AND REFRACTED WAVES

For convenience, the surrounding elastic medium and the thick-wall cylinder will be denoted as regions Nos. 1 and 2, respectively.

The incident wave, propagating in the positive X-direction is represented by

$$\begin{aligned}\phi_{(1)}^{(i)} &= \phi_0 e^{i(\alpha_1 x - \omega t)} \\ \psi_{(1)}^{(i)} &= 0\end{aligned}\tag{7}$$

where

$\phi_0$  is a measure of the amplitude

$\omega$  the circular frequency

$\alpha_1 = \omega / c_{\alpha_1}$  is the wave number of the compressional wave.

In polar coordinates

$$\phi_{(1)}^{(i)} = \phi_0 \sum_{n=0}^{\infty} \epsilon_n i^n J_n(\alpha_1 r) \cos n\theta e^{-i\omega t}\tag{8}$$

where

$J_n$  denotes the Bessel function of the first kind of order  $n$

$\epsilon_n$  is a constant so that

$$\epsilon_n = \begin{cases} 1 & n = 0 \\ 2 & n \geq 1 \end{cases}$$

Let the origin of the polar coordinates coincide with the central axis of the cylinder. The waves in regions Nos. 1 and 2 can then be expressed as follows.

REGION NO. 1

$$\phi_{(1)}^{(R)} = \sum_{n=0}^{\infty} A_n H_n^{(1)}(\alpha_1 r) \cos n\theta e^{-i\omega t} \quad (9)$$

$$\psi_{(1)}^{(R)} = \sum_{n=0}^{\infty} B_n H_n^{(1)}(\beta_1 r) \sin n\theta e^{-i\omega t} \quad (10)$$

REGION NO. 2

$$\phi_{(2)}^{(i)} = \sum_{n=0}^{\infty} M_n H_n^{(2)}(\alpha_2 r) \cos n\theta e^{-i\omega t} \quad (11)$$

$$\psi_{(2)}^{(i)} = \sum_{n=0}^{\infty} N_n H_n^{(2)}(\beta_2 r) \sin n\theta e^{-i\omega t} \quad (12)$$

$$\phi_{(2)}^{(R)} = \sum_{n=0}^{\infty} R_n H_n^{(1)}(\alpha_2 r) \cos n\theta e^{-i\omega t} \quad (13)$$

$$\psi_{(2)}^{(R)} = \sum_{n=0}^{\infty} S_n H_n^{(1)}(\beta_2 r) \sin n\theta e^{-i\omega t} \quad (14)$$

In Equations (9) through (14)

$\phi_{(1)}^{(R)} \psi_{(1)}^{(R)}$  represents the compressional and shear waves in region No. 1

$\phi_{(2)}^{(i)} \psi_{(2)}^{(i)}$  represents the inward propagating compressional and shear waves in region No. 2

$\phi_{(2)}^{(R)} \psi_{(2)}^{(R)}$  are the outgoing waves in region No. 2

$A_n, B_n, M_n, N_n, R_n$ , and  $S_n$  are expansion coefficients to be determined

$\alpha_1$  and  $\alpha_2$  are the compressional wave numbers in regions Nos. 1 and 2

$\beta_1$  and  $\beta_2$  are the shear wave numbers in regions Nos. 1 and 2 ( $\beta_1 = \frac{\omega}{c_{\beta_1}}$  etc.)

$H_n^{(1)}$  and  $H_n^{(2)}$  denote the Hankel functions of the first and second kind of order  $n$ . The Hankel function of the first kind is used for diverging waves; e.g., in Equations (9), (10), (13), and (14). The Hankel function of the second kind represents converging waves; e.g., in Equations (11) and (12).

In Equations (9) through (14) there are six undetermined coefficients. The boundary conditions which allow these coefficients to be determined are listed below.

At  $r = b$  the condition of continuity requires that

$$\begin{aligned}\tau_{rr(2)} &= \tau_{rr(1)} \\ \tau_{r\theta(2)} &= \tau_{r\theta(1)} \\ u_{r(2)} &= u_{r(1)} \\ u_{\theta(2)} &= u_{\theta(1)}\end{aligned}\tag{15}$$

At  $r = a$  the traction-free boundary implies

$$\begin{aligned}\tau_{rr(2)} &= 0 \\ \tau_{r\theta(2)} &= 0\end{aligned}\tag{16}$$

## SECTION IV

### SOLUTIONS

Where  $\tau_{rr(1), (2)}$ ,  $\tau_{r\theta(1), (2)}$ ,  $u_{r(1), (2)}$ ,  $u_{\theta(1), (2)}$  are the stresses and displacements due to the total displacement potential in regions Nos. 1 and 2; e. g.,

$$\begin{aligned}\tau_{rr(1)} &= \tau_{rr(1)} \left( \phi_{(1)} \psi_{(1)} \right) \\ \tau_{rr(2)} &= \tau_{rr(2)} \left( \phi_{(2)} \psi_{(2)} \right)\end{aligned}\tag{17}$$

where

$$\begin{aligned}\phi_{(1)} &= \phi_{(1)}^{(i)} + \phi_{(1)}^{(R)} \\ \psi_{(1)} &= \psi_{(1)}^{(R)} \\ \phi_{(2)} &= \phi_{(2)}^{(i)} + \phi_{(2)}^{(R)} \\ \psi_{(2)} &= \psi_{(2)}^{(i)} + \psi_{(2)}^{(R)}\end{aligned}\tag{18}$$

Substituting Equations (9) through (14) and (18) into Equations (5) and (6) yields the corresponding displacement and stress components in regions Nos. 1 and 2. With the time factor  $e^{-i\omega t}$  omitted, the expressions for the displacement are

$$u_{r(1)} = r^{-1} \sum_{n=0}^{\infty} \left[ \phi_0 \epsilon_n i^n \alpha_1 r J_n'(\alpha_1 r) + A_n \alpha_1 r H_n^{(1)'}(\alpha_1 r) + B_n n H_n^{(1)}(\beta_1 r) \right] \cos n\theta \quad (19)$$

$$u_{\theta(1)} = -r^{-1} \sum_{n=0}^{\infty} \left[ \phi_0 \epsilon_n i^n n J_n(\alpha_1 r) + A_n n H_n^{(1)}(\alpha_1 r) + B_n \beta_1 r H_n^{(1)'}(\beta_1 r) \right] \sin n\theta \quad (20)$$

$$u_{r(2)} = r^{-1} \sum_{n=0}^{\infty} \left[ M_n \alpha_2 r H_n^{(2)'}(\alpha_2 r) + N_n n H_n^{(2)}(\beta_2 r) + R_n \alpha_2 r H_n^{(1)'}(\alpha_2 r) + S_n n H_n^{(1)}(\beta_2 r) \right] \cos n\theta \quad (21)$$

$$u_{\theta(2)} = -r^{-1} \sum_{n=0}^{\infty} \left[ M_n n H_n^{(2)}(\alpha_2 r) + N_n \beta_2 r H_n^{(2)'}(\beta_2 r) + R_n n H_n^{(1)}(\alpha_2 r) + S_n \beta_2 r H_n^{(1)'}(\beta_2 r) \right] \sin n\theta \quad (22)$$

The expressions for the stress are

$$\tau_{rr(1)} = 2\mu_1 r^{-2} \sum_{n=0}^{\infty} (\phi_0 \epsilon_n i^n D_{nr}^{(i)} + A_{n1} D_{nr}^{(R)} + B_{n1} \mathcal{D}_{nr}^{(R)}) \cos n\theta \quad (23)$$

$$\tau_{\theta\theta(1)} = 2\mu_1 r^{-2} \sum_{n=0}^{\infty} (\phi_0 \epsilon_n i^n F_{nr}^{(i)} + A_{n1} F_{nr}^{(R)} - B_{n1} \mathcal{D}_{nr}^{(R)}) \cos n\theta \quad (24)$$

$$\tau_{r\theta(1)} = 2\mu_1 r^{-2} \sum_{n=0}^{\infty} (\phi_0 \epsilon_n i^n {}_1E_{nr}^{(i)} + A_{n1} E_{nr}^{(R)} + B_{n1} \xi_{nr}^{(R)}) \sin n\theta \quad (25)$$

$$\tau_{rr(2)} = 2\mu_2 r^{-2} \sum_{n=0}^{\infty} (M_{n2} D_{nr}^{(i)} + N_{n2} \mathcal{D}_{nr}^{(i)} + R_{n2} D_{nr}^{(R)} + S_{n2} \mathcal{D}_{nr}^{(R)}) \cos n\theta \quad (26)$$

$$\tau_{\theta\theta(2)} = 2\mu_2 r^{-2} \sum_{n=0}^{\infty} (M_{n2} F_{nr}^{(i)} - N_{n2} \mathcal{D}_{nr}^{(i)} + R_{n2} F_{nr}^{(R)} - S_{n2} \mathcal{D}_{nr}^{(R)}) \cos n\theta \quad (27)$$

$$\tau_{r\theta(2)} = 2\mu_2 r^{-2} \sum_{n=0}^{\infty} (M_{n2} E_{nr}^{(i)} + N_{n2} \xi_{nr}^{(i)} + R_{n2} E_{nr}^{(R)} + S_{n2} \xi_{nr}^{(R)}) \sin n\theta \quad (28)$$

with

$$\begin{aligned} {}_1D_{nr}^{(i)} &= (n^2 + n - \frac{1}{2} \beta_1^2 r^2) J_n(\alpha_1 r) - \alpha_1 r J_{n-1}(\alpha_1 r) \\ {}_1E_{nr}^{(i)} &= (n^2 + n) J_n(\alpha_1 r) - n \alpha_1 r J_{n-1}(\alpha_1 r) \\ {}_1F_{nr}^{(i)} &= -(n^2 + n - \alpha_1^2 r^2 + \frac{1}{2} \beta_1^2 r^2) J_n(\alpha_1 r) + \alpha_1 r J_{n-1}(\alpha_1 r) \end{aligned} \quad (29)$$

$${}_j D_{nr}^{(R)} = (n^2 + n - \frac{1}{2} \beta_j^2 r^2) H_n^{(1)}(\alpha_j r) - \alpha_j r H_{n-1}^{(1)}(\alpha_j r)$$

$${}_j E_{nr}^{(R)} = (n^2 + n) H_n^{(1)}(\alpha_j r) - n \alpha_j r H_{n-1}^{(1)}(\alpha_j r)$$

$${}_j F_{nr}^{(R)} = -(n^2 + n - \alpha_j^2 r^2 + \frac{1}{2} \beta_j^2 r^2) H_n^{(1)}(\alpha_j r) + \alpha_j r H_{n-1}^{(1)}(\alpha_j r) \quad (30)$$

$$j = 1, 2$$

$${}_2 D_{nr}^{(i)} = (n^2 + n - \frac{1}{2} \beta_2^2 r^2) H_n^{(2)}(\alpha_2 r) - \alpha_2 r H_{n-1}^{(2)}(\alpha_2 r)$$

$${}_2 E_{nr}^{(i)} = (n^2 + n) H_n^{(2)}(\alpha_2 r) - n \alpha_2 r H_{n-1}^{(2)}(\alpha_2 r)$$

$${}_2 F_{nr}^{(i)} = -(n^2 + n - \alpha_2^2 r^2 + \frac{1}{2} \beta_2^2 r^2) H_n^{(2)}(\alpha_2 r) + \alpha_2 r H_{n-1}^{(2)}(\alpha_2 r) \quad (31)$$

$${}_j \mathcal{D}_{nr}^{(R)} = -(n^2 + n) H_n^{(1)}(\beta_j r) + n \beta_j r H_{n-1}^{(1)}(\beta_j r)$$

$${}_j \mathcal{E}_{nr}^{(R)} = -(n^2 + n - \frac{1}{2} \beta_j^2 r^2) H_n^{(1)}(\beta_j r) + \beta_j r H_{n-1}^{(1)}(\beta_j r) \quad (32)$$

$$j = 1, 2$$



$${}_2\mathcal{D}_{nr}^{(i)} = -(n^2 + n) H_n^{(2)}(\beta_2 r) + n\beta_2 r H_{n-1}^{(2)}(\beta_2 r)$$

$${}_2\mathcal{E}_{nr}^{(i)} = -(n^2 + n - \frac{1}{2}\beta_2^2 r^2) H_n^{(2)}(\beta_2 r) + \beta_2 r H_{n-1}^{(2)}(\beta_2 r) \quad (33)$$

and

$$H'_n(x) = \frac{dH_n}{dx}$$

$$J'_n(x) = \frac{dJ_n}{dx} \quad (34)$$

Equations (19) through (28) are the general expression for the stresses and displacements in regions Nos. 1 and 2. To evaluate the coefficient  $A_n$ , etc., use the conditions of continuity at  $r = b$  and the boundary conditions at  $r = a$ .

At  $r = b$

$$\tau_{rr(2)} = \tau_{rr(1)}$$

$$M_{n2} D_{nb}^{(i)} + N_{n2} \mathcal{D}_{nb}^{(i)} + R_{n2} D_{nb}^{(R)} + S_{n2} \mathcal{D}_{nb}^{(R)} - \nu(A_{n1} D_{nb}^{(R)} + B_{n1} \mathcal{D}_{nb}^{(R)}) = \nu \phi_0 \epsilon_n i^n D_{nb}^{(i)} \quad (35)$$

$$\tau_{r\theta(2)} = \tau_{r\theta(1)}$$

$$M_{n2} E_{nb}^{(i)} + N_{n2} \mathcal{E}_{nb}^{(i)} + R_{n2} E_{nb}^{(R)} + S_{n2} \mathcal{E}_{nb}^{(R)} - \nu(A_{n1} E_{nb}^{(R)} + B_{n1} \mathcal{E}_{nb}^{(R)}) = \nu \phi_0 \epsilon_n i^n E_{nb}^{(i)} \quad (36)$$

$$u_{r(2)} = u_{r(1)}$$

$$M_n \alpha_2 b H_n^{(2)'} (\alpha_2 b) + N_n n H_n^{(2)} (\beta_2 b) + R_n \alpha_2 b H_n^{(1)'} (\alpha_2 b) + S_n n H_n^{(1)} (\beta_2 b) - \left[ A_n \alpha_1 b H_n^{(1)'} (\alpha_1 b) + B_n n H_n^{(1)} (\beta_1 b) \right] = \phi_0 \epsilon_n i^n \alpha_1 b J_n' (\alpha_1 b) \quad (37)$$

$$u_{\theta(2)} = u_{\theta(1)}$$

$$M_n n H_n^{(2)} (\alpha_2 b) + N_n \beta_2 b H_n^{(2)'} (\beta_2 b) + R_n n H_n^{(1)} (\alpha_2 b) + S_n \beta_2 b H_n^{(1)'} (\beta_2 b) - \left[ A_n n H_n^{(1)} (\alpha_1 b) + B_n \beta_1 b H_n^{(1)'} (\beta_1 b) \right] = \phi_0 \epsilon_n i^n n J_n (\alpha_1 b) \quad (38)$$

At  $r = a$

$$\tau_{rr(2)} = 0$$

$$M_n^{(2)} D_{na}^{(i)} + N_n^{(2)} \mathcal{D}_{na}^{(i)} + R_n^{(2)} D_{na}^{(R)} + S_n^{(2)} \mathcal{D}_{na}^{(R)} = 0 \quad (39)$$

$$\tau_{r\theta(2)} = 0$$

$$M_n^{(2)} E_{na}^{(i)} + N_n^{(2)} \mathcal{E}_{na}^{(i)} + R_n^{(2)} E_{na}^{(R)} + S_n^{(2)} \mathcal{E}_{na}^{(R)} = 0 \quad (40)$$

where  $\nu = \mu_1 / \mu_2$  is the ratio of the shear moduli of regions Nos. 1 and 2.

Thus, the coefficients  $A_n$ ,  $B_n$ ,  $M_n$ ,  $N_n$ ,  $R_n$ , and  $S_n$  are determined by the six simultaneous Equations (35) through (40).

Using matrix notation, Equations (35) through (40) can be conveniently expressed as follows

$$[a_{ij}] \{c_j\} = \{b_i\} \quad (41)$$

where

$$[a_{ij}] = \begin{bmatrix} \nu_1 D_{nb}^{(R)} & \nu_1 \mathcal{D}_{nb}^{(R)} & 2 D_{nb}^{(i)} & 2 \mathcal{D}_{nb}^{(i)} & 2 D_{nb}^{(R)} & 2 \mathcal{D}_{nb}^{(R)} \\ \nu_1 E_{nb}^{(R)} & \nu_1 \mathcal{E}_{nb}^{(R)} & 2 E_{nb}^{(i)} & 2 \mathcal{E}_{nb}^{(i)} & 2 E_{nb}^{(R)} & 2 \mathcal{E}_{nb}^{(R)} \\ \alpha_1 b H_n^{(1)'}(\alpha_1 b) & n H_n^{(1)}(\beta_1 b) & \alpha_2 b H_n^{(2)'}(\alpha_2 b) & n H_n^{(2)}(\beta_2 b) & \alpha_2 b H_n^{(1)'}(\alpha_2 b) & n H_n^{(1)}(\beta_2 b) \\ n H_n^{(1)}(\alpha_1 b) & \beta_1 b H_n^{(1)'}(\beta_1 b) & n H_n^{(2)}(\alpha_2 b) & \beta_2 b H_n^{(2)'}(\beta_2 b) & n H_n^{(1)}(\alpha_2 b) & \beta_2 b H_n^{(1)'}(\beta_2 b) \\ 0 & 0 & 2 D_{na}^{(i)} & 2 \mathcal{D}_{na}^{(i)} & 2 D_{na}^{(R)} & 2 \mathcal{D}_{na}^{(R)} \\ 0 & 0 & 2 E_{na}^{(i)} & 2 \mathcal{E}_{na}^{(i)} & 2 E_{na}^{(R)} & 2 \mathcal{E}_{na}^{(R)} \end{bmatrix} \quad (42)$$

$$\{c_j\} = \begin{pmatrix} -A_n \\ -B_n \\ M_n \\ N_n \\ R_n \\ S_n \end{pmatrix} \quad (43)$$

and

$$\{b_i\} = \phi_0 \epsilon_n i^n \begin{pmatrix} {}^\nu_1 D_{nb}^{(i)} \\ {}^\nu_1 E_{nb}^{(i)} \\ \alpha_1 b J_n'(\alpha_1 b) \\ n J_n(\alpha_1 b) \\ 0 \\ 0 \end{pmatrix} \quad (44)$$

Hence

$$\{c_i\} = \frac{[A_{ji}]}{|a|} \{b_j\} \quad (45)$$

where  $[A_{ji}]$  is the adjoint of  $[a_{ij}]$  and  $|a|$  is the determinant of  $\{a_{ij}\}$ .

## REDUCTION TO THE SIMPLE CAVITY CASE

Consider the case when the material constants of the two regions are the same:

$$\lambda_1 = \lambda_2 = \lambda$$

$$\mu_1 = \mu_2 = \mu$$

$$\rho_1 = \rho_2 = \rho$$

Therefore

$$\alpha_1 = \alpha_2 = \alpha$$

$$\beta_1 = \beta_2 = \beta$$

$$\nu = 1$$

It follows that

$${}_1D_{nr}^{(R)} = {}_2D_{nr}^{(R)}$$

$${}_1\mathcal{D}_{nr}^{(R)} = {}_2\mathcal{D}_{nr}^{(R)}$$

$${}_1E_{nr}^{(R)} = {}_2E_{nr}^{(R)}$$

$${}_1\mathcal{E}_{nr}^{(R)} = {}_2\mathcal{E}_{nr}^{(R)} \quad (46)$$

Furthermore, using the relationship between  $H_n^{(1)}$  and  $H_n^{(2)}$

$$H_n^{(1)}(x) + H_n^{(2)}(x) = 2J_n(x) \quad (47)$$

it can be shown that

$${}_2D_{nr}^{(i)} + {}_2D_{nr}^{(R)} = 2{}_1D_{nr}^{(i)} \quad (48)$$

$${}_2E_{nr}^{(i)} + {}_2E_{nr}^{(R)} = 2{}_1E_{nr}^{(i)} \quad (49)$$

Substituting Equations (46), (48), and (49) into Equation (45), the undetermined coefficients for the two potentials in region No. 1 are

$$A_n = -\phi_0 \epsilon_n i^n \frac{\begin{vmatrix} {}_1D_{na}^{(i)} & {}_1D_{na}^{(R)} \\ {}_1E_{na}^{(i)} & {}_1E_{na}^{(R)} \end{vmatrix}}{\begin{vmatrix} {}_1D_{na}^{(R)} & {}_1D_{na}^{(R)} \\ {}_1E_{na}^{(R)} & {}_1E_{na}^{(R)} \end{vmatrix}} \quad (50)$$

and

$$B_n = -\phi_0 \epsilon_n i^n \frac{\begin{vmatrix} 1 D_{na}^{(R)} & 1 D_{na}^{(i)} \\ 1 E_{na}^{(R)} & 1 E_{na}^{(i)} \end{vmatrix}}{\begin{vmatrix} 1 D_{na}^{(R)} & 1 \mathcal{D}_{na}^{(R)} \\ 1 E_{na}^{(R)} & 1 \mathcal{E}_{na}^{(R)} \end{vmatrix}} \quad (51)$$

These expressions for  $A_n$  and  $B_n$  are the same as those in reference 1. The remaining coefficients  $M_n$ ,  $N_n$ ,  $R_n$ , and  $S_n$  are

$$\begin{aligned} M_n &= \frac{1}{2} \phi_0 \epsilon_n i^n \\ N_n &= 0 \\ S_n &= B_n \\ R_n &= \frac{1}{2} \phi_0 \epsilon_n i^n + A_n \end{aligned} \quad (52)$$

Thus the shear potential in region No. 2 is obviously the same as that in region No. 1. The total scalar potential in region No. 2 is

$$\begin{aligned}
\phi_{(2)} &= \phi_{(2)}^{(i)} + \phi_{(2)}^{(R)} = \sum_{n=0}^{\infty} \frac{1}{2} \phi_0 \epsilon_n i^n H_n^{(2)}(\alpha r) \cos n\theta + \\
&\quad \left( \frac{1}{2} \phi_0 \epsilon_n i^n + A_n \right) H_n^{(1)}(\alpha r) \cos n\theta \\
&= \sum_{n=0}^{\infty} \left[ \phi_0 \epsilon_n i^n J_n(\alpha r) + A_n H_n^{(1)}(\alpha r) \right] \cos n\theta \tag{53}
\end{aligned}$$

which is identical to the scalar potential in region No. 1. Therefore, the problem is reduced to a simple cavity case.



## SECTION V

### NUMERICAL RESULTS AND DISCUSSIONS

Numerical results are obtained for  $\tau_{rr(1)}$  and  $\tau_{\theta\theta(1)}$  at  $r = b$  and  $\tau_{\theta\theta(2)}$  at  $r = a$ . This is accomplished by summing the series in Equations (23), (24), and (27), respectively.

For reasons of expediency, the expressions for the stresses are non-dimensionalized. The important nondimensional parameters can then be properly identified, and the effects of these parameters on the stresses in the cylinder as well as in the surrounding medium can be evaluated.

To nondimensionalize these expressions, recall the expression for the incident wave given in Equation (7) as

$$\phi_{(1)}^{(i)} = \phi_0 e^{i(\alpha_1 x - \omega t)}$$

Therefore, the stress in the incident wave in the direction of propagation is

$$\tau_{xx} = -(\lambda_1 + 2\mu_1) \alpha_1^2 \phi_0 e^{i(\alpha_1 x - \omega t)} \quad (54)$$

It follows that  $-(\lambda_1 + 2\mu_1) \alpha_1^2 \phi_0$  is the stress amplitude of the incident wave. Denote it as

$$\tau_0 = -(\lambda_1 + 2\mu_1) \alpha_1^2 \phi_0 = -\beta_1^2 \mu_1 \phi_0 \quad (55)$$

Thus, Equations (23), (24), and (27) can be nondimensionalized by dividing through by  $\tau_0$  as shown below:

$$\tau_{rr(1)}^* = \frac{\tau_{rr(1)}}{\tau_0} = - \frac{2}{\phi_0 \beta_1^2 r^2} \sum_{n=0}^{\infty} (\phi_0 \epsilon_n i^n D_{nr}^{(i)} + A_{n1} D_{nr}^{(R)} + B_{n1} \mathcal{D}_{nr}^{(R)}) \cos n\theta \quad (56)$$

$$\tau_{\theta\theta(1)}^* = \frac{\tau_{\theta\theta(1)}}{\tau_0} = - \frac{2}{\phi_0 \beta_1^2 r^2} \sum_{n=0}^{\infty} (\phi_0 \epsilon_n i^n F_{nr}^{(i)} + A_{n1} F_{nr}^{(R)} - B_{n1} \mathcal{D}_{nr}^{(R)}) \cos n\theta \quad (57)$$

$$\tau_{\theta\theta(2)}^* = \frac{\tau_{\theta\theta(2)}}{\tau_0} = - \frac{2}{\phi_0 \nu \beta_1^2 r^2} \sum_{n=0}^{\infty} (M_{n2} F_{nr}^{(i)} - N_{n2} \mathcal{D}_{nr}^{(i)} + R_{n2} F_{nr}^{(R)} - S_{n2} \mathcal{D}_{nr}^{(R)}) \cos n\theta \quad (58)$$

where  $\nu = \mu_1/\mu_2$  is the ratio of shear moduli of regions Nos. 1 and 2.

By letting  $r = b$  in Equations (56) and (57), and  $r = a$  in Equation (58) we have, for example,

$$\tau_{rr}^* = - \frac{2}{\phi_0 \beta_1^2 b^2} \sum_{n=0}^{\infty} (\phi_0 \epsilon_n i^n D_{nb}^{(i)} + A_{n1} D_{nb}^{(R)} + B_{n1} \mathcal{D}_{nb}^{(R)}) \cos n\theta \quad (59)$$

$r = b$

with similar expressions for Equations (57) and (58).

By examining the arguments in  $D_{nb}^{(i)} \dots$  etc., that are associated with various terms in Equation (59) etc., it is found that five essential nondimensional parameters exist. These are

$$\nu = \mu_1 / \mu_2$$

$$\gamma = \frac{\alpha_2}{\alpha_1} = \frac{c_{\alpha_1}}{c_{\alpha_2}}$$

ratio of the dilatational phase velocities of regions Nos. 1 and 2

$$k_1^2 = \left( \frac{\beta_1}{\alpha_1} \right)^2 = \left( \frac{c_{\alpha_1}}{c_{\beta_1}} \right)^2 = \frac{2(1 - \sigma_1)}{1 - 2\sigma_1}$$

ratio of dilatational phase velocities to distortional phase velocities in regions Nos. 1

$$k_2^2 = \left( \frac{\beta_2}{\alpha_2} \right)^2 = \left( \frac{c_{\alpha_2}}{c_{\beta_2}} \right)^2 = \frac{2(1 - \sigma_2)}{1 - 2\sigma_2}$$

and 2

$$\eta = b/a$$

ratio of outer radius to inner radius of the cylinder

By substituting these expressions into Equation (59) ... etc., the final expressions for  $\tau_{rr}^*_{(1)}$   $\tau_{\theta\theta}^*_{(1)}$  at  $r = b$  and  $\tau_{\theta\theta}^*_{(2)}$  at  $r = a$  are obtained

$$\tau_{rr}^*_{(1)} = - \frac{2}{\phi_0^2 k_1^2 \eta^2 (\alpha_1 a)^2} \sum_{n=0}^{\infty} (\phi_0 \epsilon_n i^n {}_1D_{nb}^{(i)} + A_{n1} D_{nb}^{(R)} + B_{n1} \mathcal{D}_{nb}^{(R)}) \cos n\theta$$

(60)

$$\tau_{\theta\theta}^*_{(1)} = - \frac{2}{\phi_0^2 k_1^2 \eta^2 (\alpha_1 a)^2} \sum_{n=0}^{\infty} (\phi_0 \epsilon_n i^n {}_1F_{nb}^{(i)} + A_{n1} F_{nb}^{(R)} - B_{n1} \mathcal{D}_{nb}^{(R)}) \cos n\theta$$

(61)

$$\tau_{\theta\theta}^*_{(2)} = - \frac{2}{\phi_0^2 k_1^2 (\alpha_1 a)^2} \sum_{n=0}^{\infty} (M_{n2} F_{na}^{(i)} - N_{n2} \mathcal{D}_{na}^{(i)} + R_{n2} F_{na}^{(R)} - S_{n2} \mathcal{D}_{na}^{(R)}) \cos n\theta$$

(62)

The expressions for  ${}_1D_{nb}^{(i)} \dots \mathcal{D}_{na}^{(R)}$  can be expressed in terms of the incident dimensionless wave number  $(\alpha_1 a)$  and the parameters defined above.

For example

$${}_1D_{nb}^{(i)} = (n^2 + n - \frac{1}{2} \beta_1^2 b^2) J_n(\alpha_1 b) - \alpha_1 b J_{n-1}(\alpha_1 b)$$

$$= \left[ n^2 + n - \frac{1}{2} k_1^2 \eta^2 (\alpha_1 a)^2 \right] J_n(\eta \alpha_1 a) - \eta \alpha_1 a J_{n-1}(\eta \alpha_1 a)$$

(63)

It is apparent that the stresses in the cylinder as well as in the surrounding elastic medium are not only a function of the incident wave frequency, but also depend on the four physical parameters of the two media and a geometrical parameter of the cylinder.

By restoring  $e^{-i\omega t}$ , Equations (60) through (62) have the form

$$(R + iI) e^{-i\omega t} = (R^2 + I^2)^{1/2} e^{-i(\omega t - \delta)} \quad (64)$$

The real part represents the stresses at  $t = 0$  and the imaginary part gives the stresses at  $t = T/4$ , where  $T = 2\pi/\omega$  is the period of the incident wave. The absolute values  $(R^2 + I^2)^{1/2}$  correspond to the maximum values of  $\tau_{rr(1)}^*$ ,  $\tau_{\theta\theta(1)}^*$  and  $\tau_{\theta\theta(2)}^*$  which occur in the interval  $t = 0$  to  $t = T/4$ .

The phase angles are given by  $\delta = \arctan I/R$ .

Numerical results are obtained for two cases corresponding to a soft and a stiff cylindrical lining.

#### SOFT CYLINDRICAL LINING

In this case the dimensionless physical parameters are assumed to be

$$\nu = 2.90$$

$$\gamma = 1.50$$

$$\sigma_1 = .25$$

$$\sigma_2 = .20$$

$$\eta = 1.05, 1.1 \text{ and } 1.20$$

The results are shown in Figs. 2 through 6.

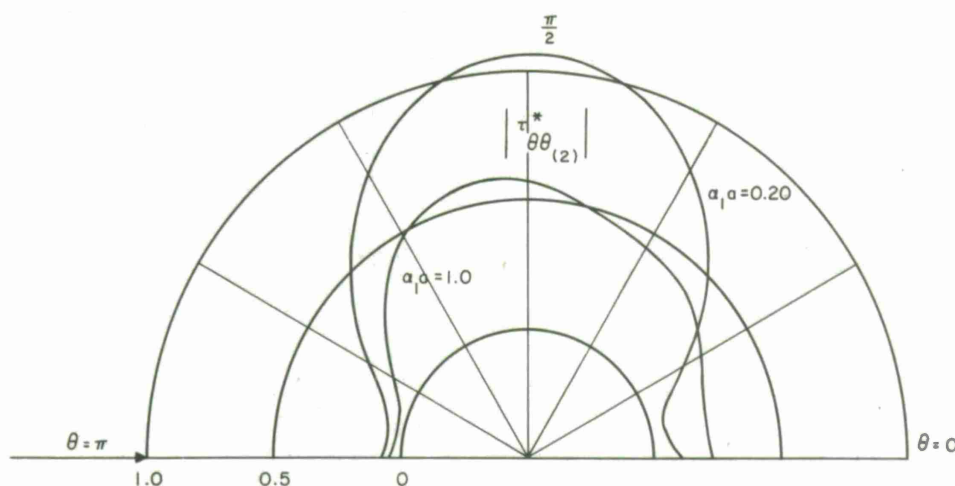


Fig. 2 Distribution of Normalized Tangential Stress at  $r = a$  and  $\eta = 1.1$  (Case I)

Fig. 2 shows the angular distribution of  $|\tau^*_{\theta\theta(2)}|$  for  $\eta = 1.1$  at  $r = a$  for two wave numbers:  $\alpha_1 a = .20$  and  $\alpha_1 a = 1.0$ . At the low wave number, the distribution is nearly symmetrical with respect to the  $y$  axis; at  $\alpha_1 a = 1.0$ , the peak stress is shifted toward the incident side of the cylinder. This is also found in reference 2.

Fig. 3 shows the stresses as a function of  $\alpha_1 a$  and  $\eta$  at  $t = 0$  and  $t = T/4$ . For this case, an increase in  $\eta$  does not change the stress appreciably; in fact, as  $\eta$  increases, the stress also increases.

Fig. 4 shows the stresses at  $\theta = \pi/2$ ,  $\pi$  and  $r = a$  for  $.10 \leq \alpha_1 a \leq 2.0$ . The maximum stresses at  $\pi/2$  occur at  $\alpha_1 a \approx .25$ ; this agrees with the results of reference 1.

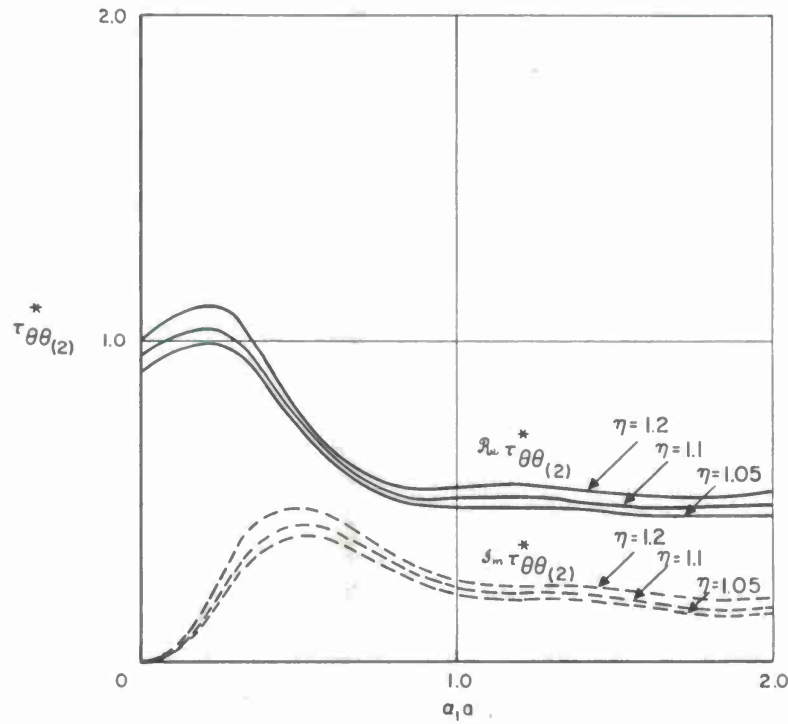


Fig. 3 Normalized Real and Imaginary Tangential Stresses at  $r = a$ ,  $\theta = \frac{\pi}{2}$  for Various  $\eta$  (Case I)

Figs. 5 and 6 show the effects of  $\eta$  on the stresses in medium (1) at  $r = b$ . It is seen that  $|\tau_{\theta\theta}^*_{(1)}|$  at  $\pi/2$  decreases as  $\eta$  increases, with the peak value again occurring at  $\alpha_1 a \approx .25$ . On the other hand,  $|\tau_{rr}^*_{(1)}|$  in all cases increases as  $\eta$  increases. This is apparent since the rigidity of the cylinder is increased as  $\eta$  increases; hence, higher radial stresses are produced at the boundary.



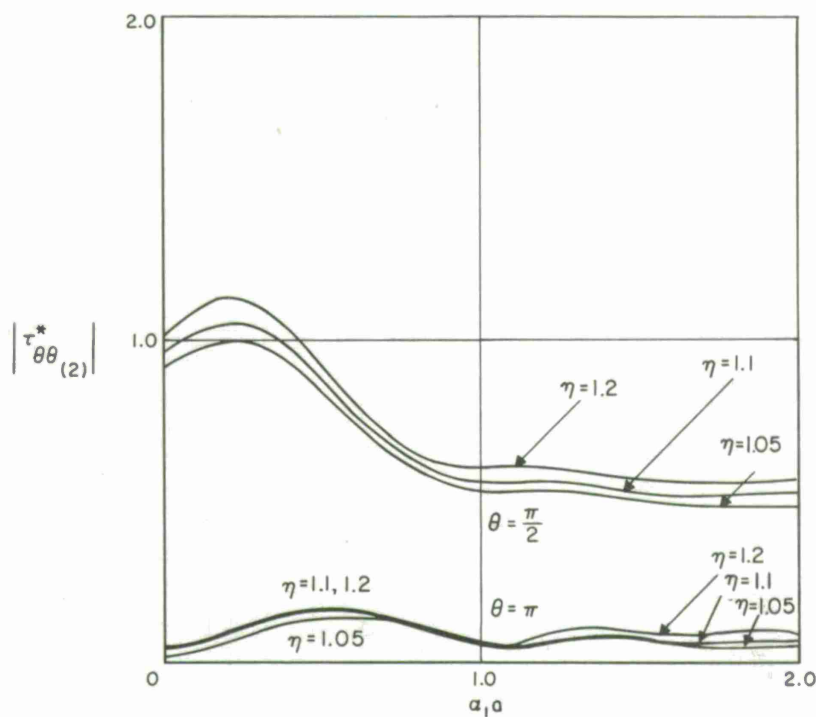


Fig. 4 Normalized Tangential Stress at  $r = a$ ,  $\theta = \frac{\pi}{2}, \pi$  for Various  $\eta$  (Case I)

#### STIFF CYLINDRICAL LINING

The dimensionless parameters are assumed to be

$$\nu = .31$$

$$\gamma = .70$$

$$\sigma_1 = .25$$

$$\sigma_2 = .30$$

$$\eta = 1.05, 1.1 \text{ and } 1.2$$

The results for this case (Case II) are shown in Figs. 7 through 11.



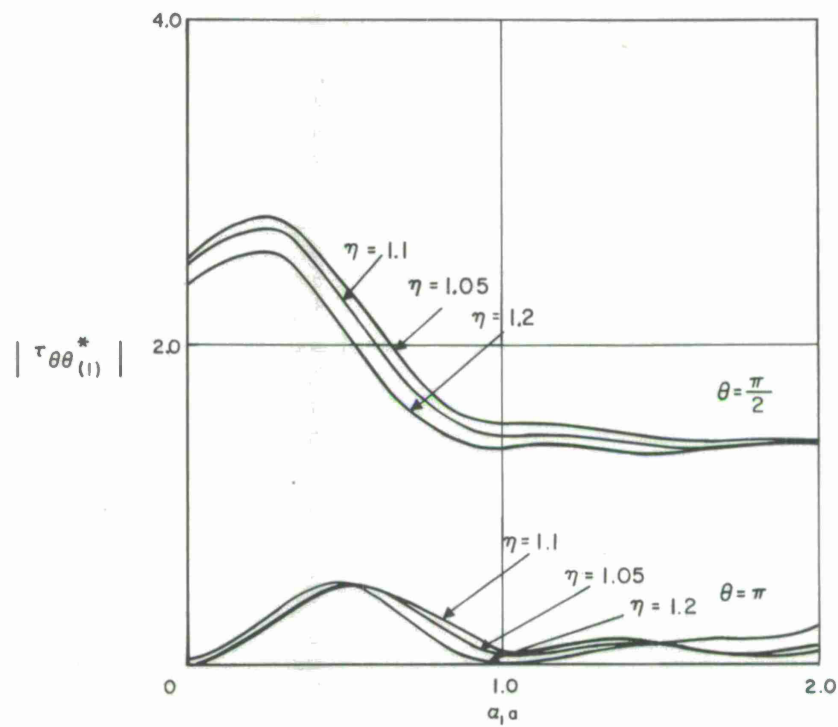


Fig. 5 Normalized Tangential Stress at  $r = b$ ,  $\theta = \frac{\pi}{2}, \pi$  for Various  $\eta$  (Case I)

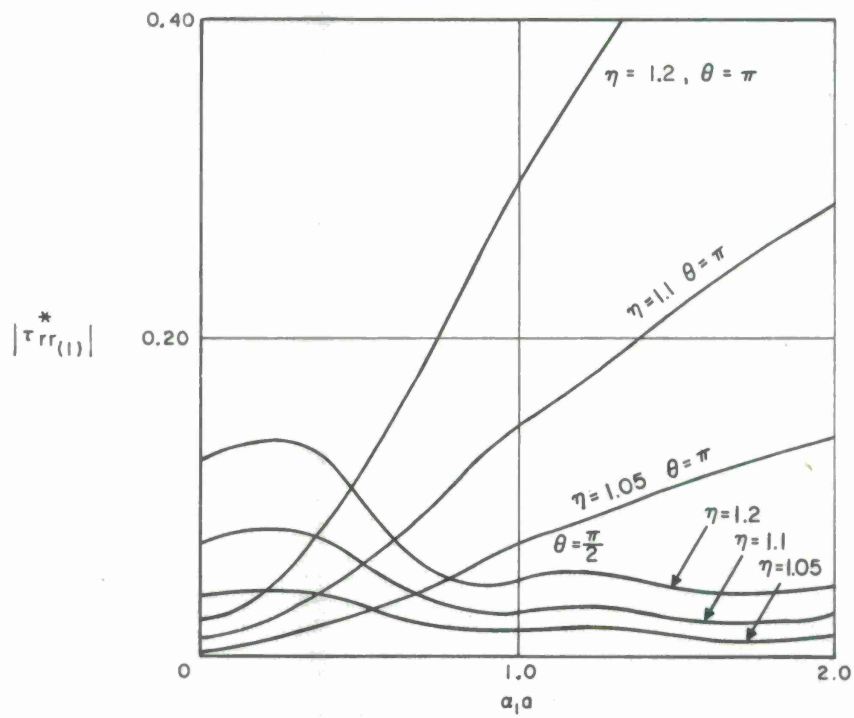


Fig. 6 Normalized Radial Stress at  $r = b$ ,  $\theta = \frac{\pi}{2}, \pi$  for Various  $\eta$  (Case I)

Fig. 7 shows the angular distribution of  $|\tau_{\theta\theta}^*|$  for  $\eta = 1.1$  at  $r = a$  for two wave numbers;  $\alpha_1 a = .20$  and  $\alpha_1 a = 1.0$ . Note that the shifting exhibited in the case of the soft cylindrical lining (Case I) also occurs in this case. The magnitude of the stresses are, however, much higher.

Fig. 9 shows the effects of  $\eta$  on stresses in the cylinder. In this case, the stress decreases as  $\eta$  increases; this is in contrast to the previous soft cylinder case.

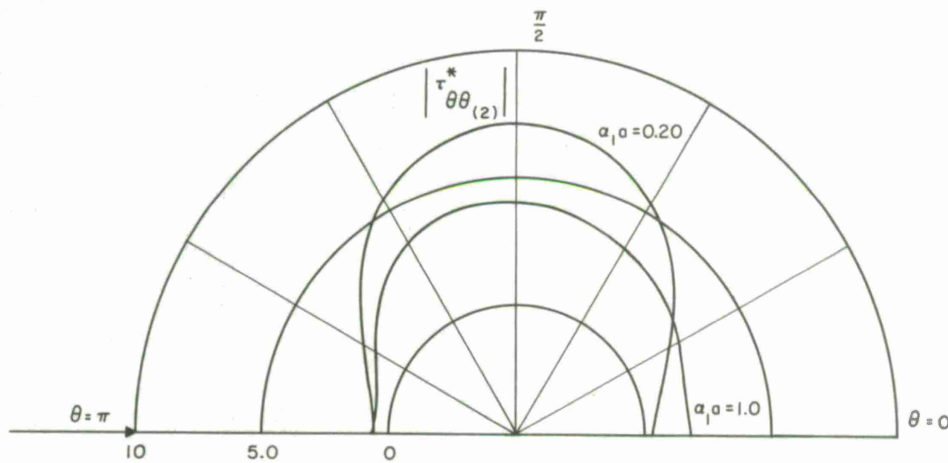


Fig. 7 Distribution of Normalized Tangential Stress at  $r = a$  and  $\eta = 1.1$  (Case II)

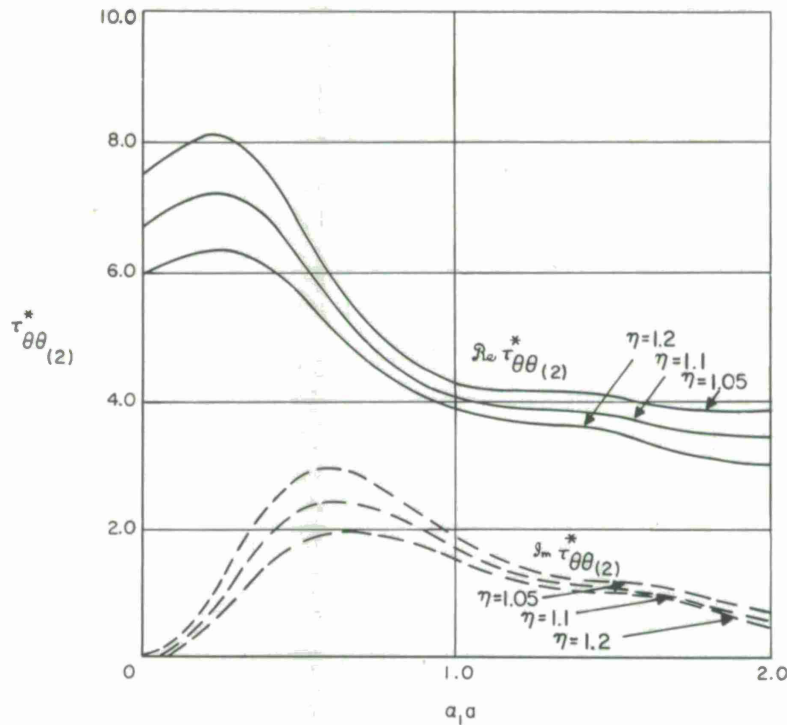


Fig. 8 Normalized Real and Imaginary Tangential Stresses at  $r = a$ ,  $\theta = \frac{\pi}{2}$ , for Various  $\eta$  (Case II)

Figs. 10 and 11 show the effects of  $\eta$  on the stress in region No. 1. The general trend is the same as in (Case I).

The contrast in the results for the stiff lining and the soft lining should be emphasized. Both cases exhibit the tendency for stress to concentrate at large wave numbers on the incident side of the cylinder; however, the tangential stress in the lining is significantly higher in the case of the stiff liner. The wave numbers for maximum stress at  $\theta = \pi/2$  and  $\pi$  are approximately the same for both the stiff and the soft lining, but the effects of increased lining thickness on tangential stress are opposite in the two cases. The stress decreases with thickness for the stiff liner, but increases with thickness in the case of the soft liner.

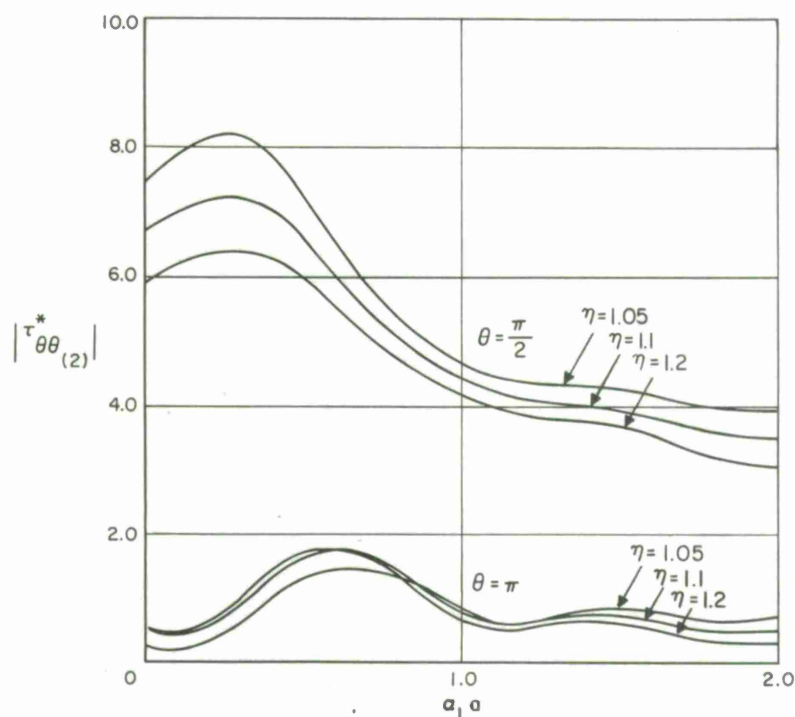


Fig. 9 Maximum Normalized Tangential Stress at  $r = a$ ,  $\theta = \frac{\pi}{2}, \pi$  for Various  $\eta$  (Case II)

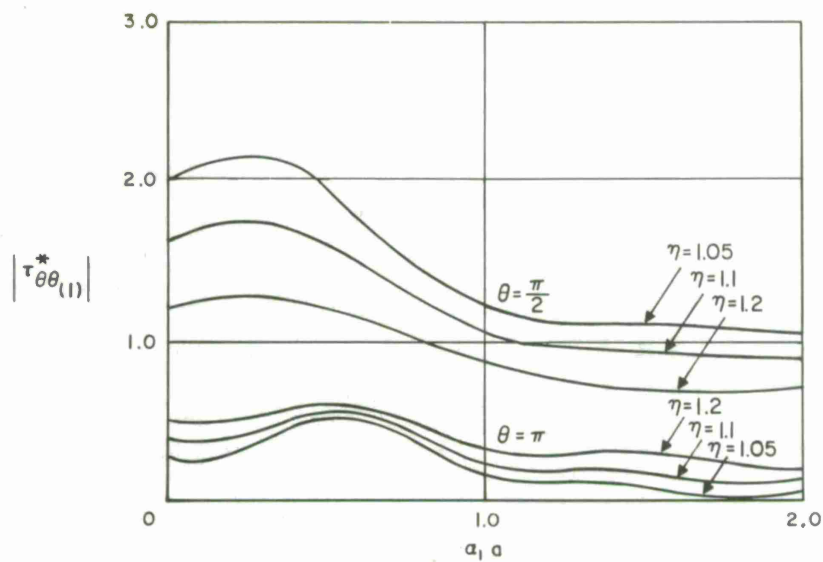


Fig. 10 Normalized Tangential Stress in Medium (1) at  $r = b$ ,  $\theta = \frac{\pi}{2}, \pi$  for Various  $\eta$  (Case II)

Other points of contrast are:

- (a) The variance of tangential stress in the cylinder with thickness is less in the case of the soft liner.
- (b) The tangential stress in the infinite medium at the boundary of the lining is less in the case of the stiff lining. In addition, increasing thickness markedly reduces this stress, significantly more so than for the soft lining.
- (c) The radial stresses in the infinite medium at the boundary of the lining are much less in the case of the soft lining. In both cases, this stress reduces rapidly with decreasing thickness.

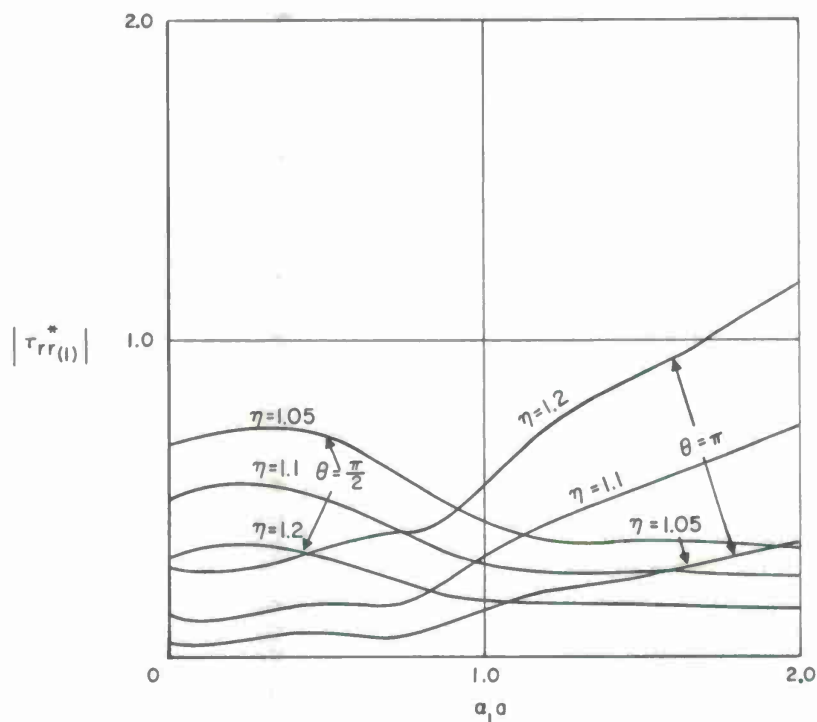
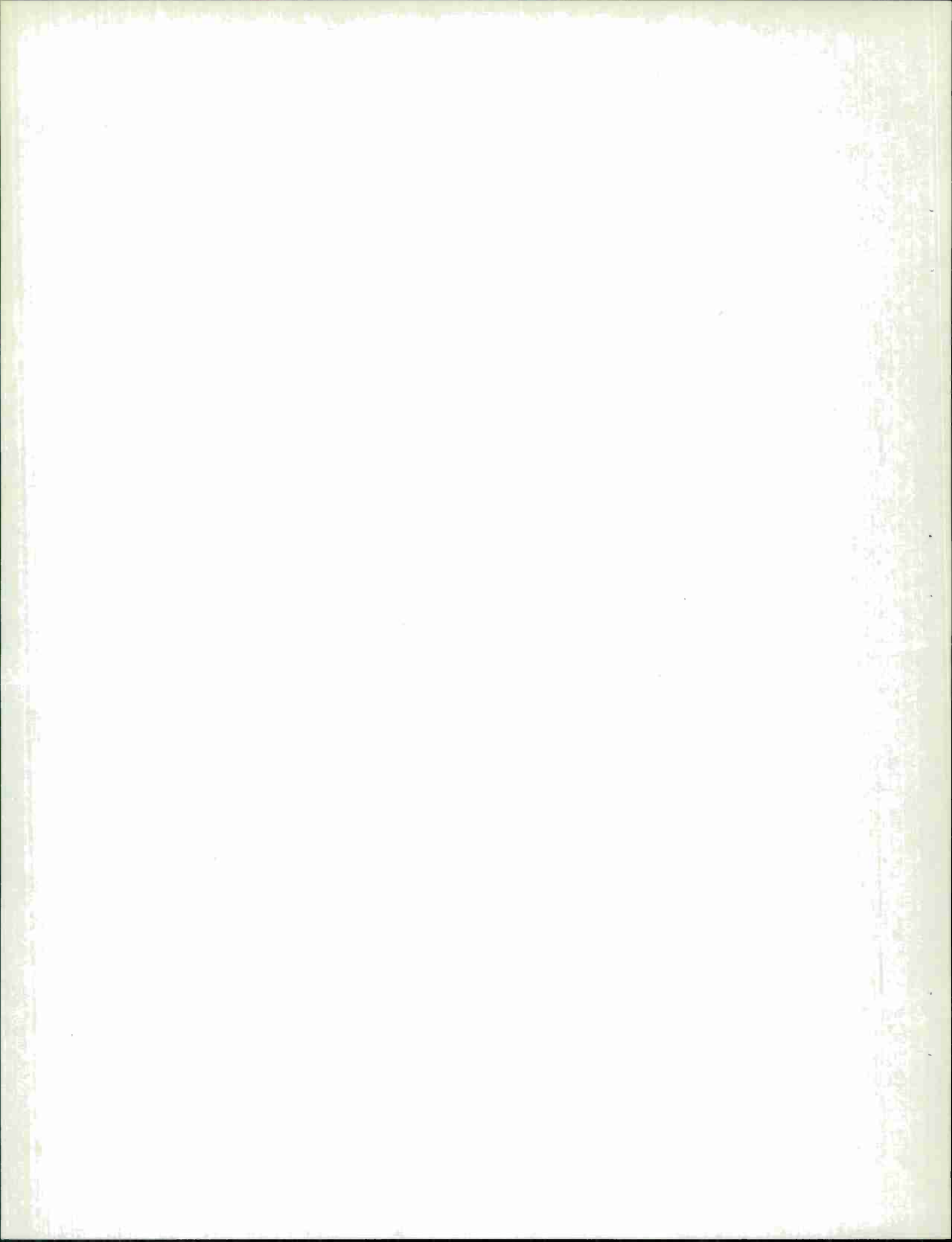


Fig. 11 Normalized Radial Stress in Medium (I) at  
 $r = b, \theta = \frac{\pi}{2}, \pi$  for Various  $\eta$  (Case II)



## APPENDIX

### THE EXACT TWO-DIMENSIONAL SOLUTION FOR AN ELASTIC CYLINDRICAL LINING IN AN INFINITE ELASTIC MEDIUM UNDER BIAXIAL COMPRESSIVE LOADING

This appendix derives expressions for the stresses in an elastic cylindrical lining in an infinite elastic medium under biaxial compressive forces. This static solution represents the infinite wave length (zero frequency) solution to which the traveling wave solution must converge in the limit. This is accomplished by using a superposition approach in which the compatibility equations for the stress and displacement at the interface between the lining and the infinite medium are expressed in terms of unknown radial and tangential tractions assumed to act on the lining and the boundary of the infinite medium. The solution derived is a plane strain solution.

#### STATIC STRESS AND DISPLACEMENT OF AN UNLINED CYLINDRICAL CAVITY BOUNDARY DUE TO THE BIAXIAL COMPRESSIVE FIELD

$$\tau_{xx} = -\tau_0, \tau_{yy} = \epsilon \tau_0$$

Consider an infinite elastic medium with a cylindrical cavity of radius "b" under biaxial compression  $\tau_{xx} = -\tau_0, \tau_{yy} = \epsilon \tau_0$  where  $\epsilon = -\sigma_1 / 1 - \sigma_1$  (Fig. 12). The subscript 1 is used to identify the infinite medium as opposed to the lining considered below. The displacements at the cavity boundary due to the biaxial compressive field<sup>[7]</sup> are

$$u'_{r(1)} = -\frac{\tau_0 b}{E_1} (1 + \sigma_1) \left[ 1 + 2(1 - 2\sigma_1) \cos 2\theta \right] \quad (65)$$

$r = b$



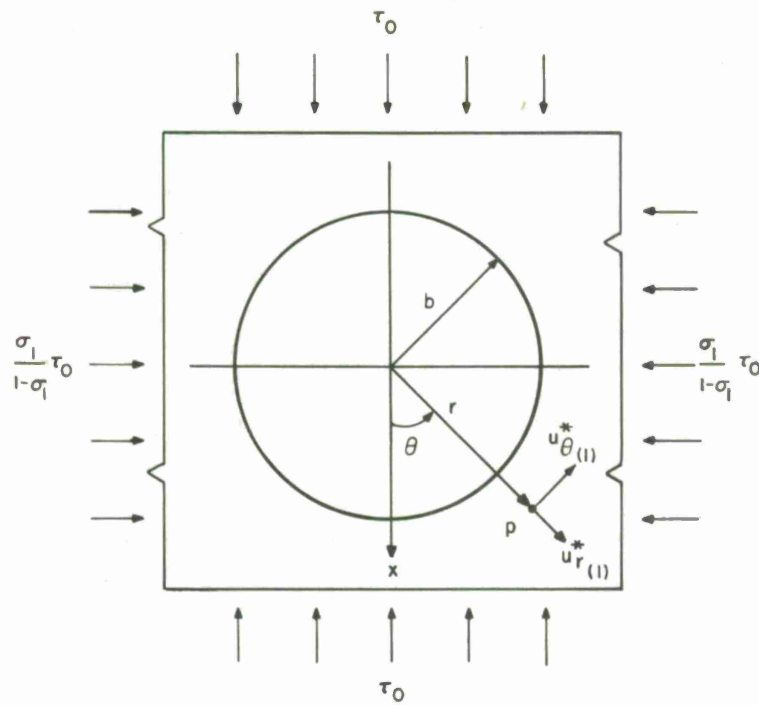


Fig. 12 Unlined Cavity

$$u'_{\theta(1)} = \frac{2\tau_0 b}{E_1} \left[ (1 - 2\sigma_1)(1 + \sigma_1) \sin 2\theta \right] \quad (66)$$

$r = b$

Reference 8 gives the tangential stress at the boundary of the cavity due to the biaxial compressive field; i.e.,

$$\tau'_{\theta\theta(1)} = \frac{\tau_0}{1 - \sigma_1} \left[ -1 + (2 - 4\sigma_1) \cos 2\theta \right] \quad (67)$$

$r = b$

The radial and shear stresses vanish at the boundary.



STATIC STRESS AND DISPLACEMENT OF AN UNLINED CAVITY BOUNDARY  
DUE TO APPLIED BOUNDARY TRACTIONS  $\bar{\tau}_{rr(1)}$  AND  $\bar{\tau}_{r\theta(1)}$

Consider an infinite elastic medium with a cylindrical cavity of radius "b" under action of applied radial and tangential boundary tractions  $\bar{\tau}_{rr(1)}$  and  $\bar{\tau}_{r\theta(1)}$ , respectively. Let

$$\bar{\tau}_{rr(1)} = \bar{\tau}_{rr0} + \bar{\tau}_{rr1} \cos 2\theta$$

$$\bar{\tau}_{r\theta(1)} = \bar{\tau}_{r\theta1} \sin 2\theta \quad (68)$$

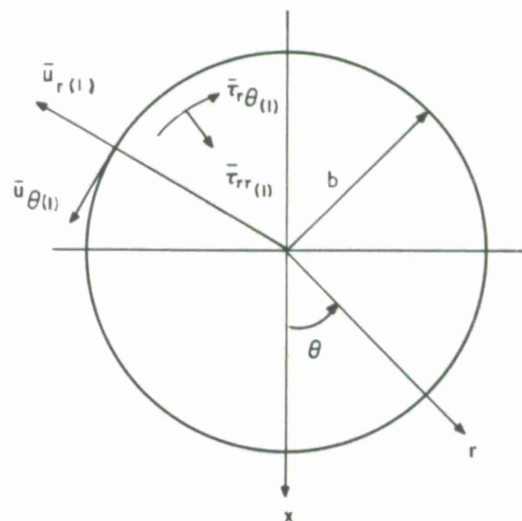


Fig. 13 Boundary Tractions - Unlined Cavity

where  $\bar{\tau}_{rr_0}$ ,  $\bar{\tau}_{rr_1}$ , and  $\bar{\tau}_{r\theta_1}$  are arbitrary constants. Fig. 13 illustrates the problem. At the boundary ( $r = b$ ) displacements and stresses are

$$\bar{u}_{r(1)} = -\frac{\bar{\tau}_{rr_0}}{E_1} (1 + \sigma_1) + \frac{2(1 + \sigma_1)b}{E_1} \left[ -\bar{\tau}_{rr_1} \left( \frac{5}{6} - \sigma_1 \right) + \bar{\tau}_{r\theta_1} \left( \frac{2}{3} - \sigma_1 \right) \right] \cos 2\theta$$

$r = b$

(69)

$$\bar{u}_{\theta(1)} = -\frac{(1 + \sigma_1)b}{E_1} \left[ -\bar{\tau}_{rr_1} \left( \frac{4}{3} - 2\sigma_1 \right) + \bar{\tau}_{r\theta_1} \left( \frac{5}{3} - 2\nu \right) \right] \sin 2\theta$$

$r = b$

(70)

and

$$\bar{\tau}_{rr(1)} = \bar{\tau}_{rr_0} + \bar{\tau}_{rr_1} \cos 2\theta$$

$r = b$

$$\bar{\tau}_{\theta\theta(1)} = -\bar{\tau}_{rr_0} + (\bar{\tau}_{rr_1} - 2\bar{\tau}_{r\theta_1}) \cos 2\theta$$

$r = b$

$$\bar{\tau}_{r\theta(1)} = \bar{\tau}_{r\theta_1} \sin 2\theta$$

$r = b$

(71)

# STATIC DISPLACEMENTS AND STRESSES IN AN ELASTIC CYLINDRICAL LINER UNDER ARBITRARY BOUNDARY TRACTIONS

The generalized stress function and stresses for plane strain in cylindrical coordinates (Fig. 14) are

$$\psi_{(2)}(r, \theta) = C_1 r^2 \ln r + C_2 r^2 + C_3 \ln r + C_4 + \left( C_5 r^2 + C_6 r^4 + \frac{C_7}{r^2} + C_8 \right) \cos 2\theta \quad (72)$$

$$\tau_{rr(2)} = C_1 (1 + 2 \ln r) + 2C_2 + \frac{C_3}{r^2} - \left( 2C_5 + \frac{6C_7}{r^4} + \frac{4C_8}{r^2} \right) \cos 2\theta \quad (73)$$

$$\tau_{\theta\theta(2)} = C_1 (3 + 2 \ln r) + 2C_2 - \frac{C_3}{r^2} + \left( 2C_5 + 12C_6 r^2 + \frac{6C_7}{r^4} \right) \cos 2\theta \quad (74)$$

$$\tau_{r\theta(2)} = \left( 2C_5 + 6C_6 r^2 - \frac{6C_7}{r^4} - \frac{2C_8}{r^2} \right) \sin 2\theta \quad (75)$$

where the subscript 2 identifies the elastic lining as opposed to the elastic infinite medium.

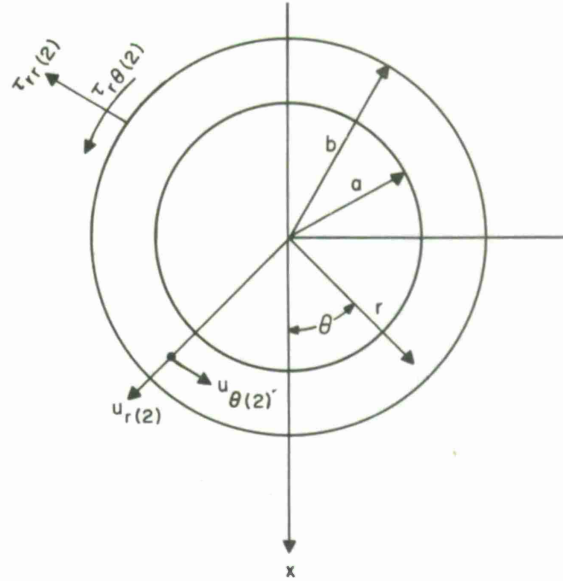


Fig. 14 Elastic Cylinder with Boundary Traction

The stress-displacement relationships are

$$\epsilon_{rr(2)} = \frac{\partial u_{r(2)}}{\partial r} = \frac{1 + \sigma_2}{E_2} \left[ (1 - \sigma_2) \tau_{rr(2)} - \sigma_2 \tau_{\theta\theta(2)} \right]$$

$$\epsilon_{\theta\theta(2)} = \frac{u_{r(2)}}{r} + \frac{1}{r} \frac{\partial u_{\theta(2)}}{\partial \theta} = \frac{1 + \sigma_2}{E_2} \left[ (1 - \sigma_2) \tau_{\theta\theta(2)} - \sigma_2 \tau_{rr(2)} \right] \quad (76)$$

Integrating these equations for  $u_{r(2)}$  and  $u_{\theta(2)}$  and neglecting rigid body motions, we obtain

$$u_{r(2)} = \frac{1 + \sigma_2}{E_2} \left\{ (1 - 2\sigma_2) \left[ C_1 (2r \ln r - r) + 2C_2 r \right] - \frac{C_3}{r} - 2\sigma_2 C_1 r \right. \\ \left. - \left[ -(1 - \sigma_2) 4 \frac{C_8}{r} + 4\sigma_2 C_6 r^3 - \frac{2C_7}{r^3} + 2C_5 r \right] \cos 2\theta \right\} \quad (77)$$

$$u_{\theta(2)} = \frac{1 + \sigma_2}{E_2} \left\{ 4(1 - \sigma_2) C_1 r \theta + \frac{1}{2} r \left[ (12 - 8\sigma_2) C_6 r^2 + 4C_5 \right. \right. \\ \left. \left. + \frac{4C_7}{r^4} - \frac{4(1 - 2\sigma_2)C_8}{r^2} \right] \sin 2\theta \right\} \quad (78)$$

In the  $u_{\theta(2)}$  equations, note that to assure periodicity in  $\theta$ , it is necessary that

$$C_1 = 0 \quad (79)$$

#### APPLICATION OF BOUNDARY AND COMPATABILITY EQUATIONS TO DETERMINE UNKNOWN COEFFICIENTS

The system of equations developed above contains nine unknowns; i. e.,  $C_2, C_3, C_5, C_6, C_7, C_8, \bar{\tau}_{rr0}, \bar{\tau}_{rr1},$  and  $\bar{\tau}_{r\theta1}$ . Evaluation of these constants entails use of the boundary conditions at  $r = a$  and the stress and displacement compatibility requirements at  $r = b$ .

Boundary Conditions at  $r = a$

$$\tau_{rr(2)} = 2C_2 + \frac{C_3}{a^2} - \left( 2C_5 + \frac{6C_7}{a^4} + \frac{4C_8}{a^2} \right) \cos 2\theta = 0$$

$r = a$

$$\tau_{r\theta(2)} = \left( 2C_5 + 6C_6 a^2 - \frac{6C_7}{a^4} - \frac{2C_8}{a^2} \right) \sin 2\theta = 0$$

$r = a$

(80)

Therefore

$$2C_2 + \frac{C_3}{a^2} = 0$$
(81)

$$2C_5 + \frac{6C_7}{a^4} + \frac{4C_8}{a^2} = 0$$
(82)

$$2C_5 + 6C_6 a^2 - \frac{6C_7}{a^4} - \frac{2C_8}{a^2} = 0$$
(83)

Stress Compatability Relations at  $r = b$

$$\tau_{rr(2)} = 2C_2 + \frac{C_3}{b^2} - \left( 2C_5 + \frac{6C_7}{b^4} + \frac{4C_8}{b^2} \right) \cos 2\theta = \bar{\tau}_{rr0} + \bar{\tau}_{rr1} \cos 2\theta$$

$r = b$

$$\tau_{r\theta(2)} = \left( 2C_5 + 6C_6 b^2 - \frac{6C_7}{b^4} - \frac{2C_8}{b^2} \right) \sin 2\theta = \bar{\tau}_{r\theta1} \sin 2\theta$$

$r = b$

(84)

Therefore

$$2C_2 + \frac{C_3}{b^2} = \bar{\tau}_{rr0} \quad (85)$$

$$-\left(2C_5 + \frac{6C_7}{b^4} + \frac{4C_8}{b^2}\right) = \bar{\tau}_{rr1} \quad (86)$$

$$2C_5 + 6C_6 b^2 - \frac{6C_7}{b^4} - \frac{2C_8}{b^2} = \bar{\tau}_{r\theta} \quad (87)$$

#### Displacement Compatibility Relations at $r = b$

$$\begin{matrix} u_{r(2)} \\ r=b \end{matrix} = \begin{matrix} u_{r(1)} \\ r=b \end{matrix} = \begin{matrix} u'_{r(1)} \\ r=b \end{matrix} + \begin{matrix} \bar{u}_{r(1)} \\ r=b \end{matrix}$$

$$\begin{matrix} u_{\theta(2)} \\ r=b \end{matrix} = \begin{matrix} u_{\theta(1)} \\ r=b \end{matrix} = \begin{matrix} u'_{\theta(1)} \\ r=b \end{matrix} + \begin{matrix} \bar{u}_{\theta(1)} \\ r=b \end{matrix} \quad (88)$$

Using Equations (65), (66), (69), (70), (77), and (78), there results

$$\frac{1 + \sigma_2}{E_2} \left[ (1 - 2\sigma_2)(2C_2 b) - \frac{C_3}{b} \right] = - \frac{\tau_0 b}{E_1} (1 + \sigma_1) - \frac{\bar{\tau}_{rr0} b}{E_1} (1 + \sigma_1) \quad (89)$$

$$\begin{aligned}
& - \frac{(1 + \sigma_2)}{E_2} \left[ 4\sigma_2 C_6 b^3 + 2C_5 b - \frac{2C_7}{b^3} - (1 - \sigma_2) \frac{4C_8}{b} \right] = - \frac{2\tau_0 b}{E_1} (1 - 2\sigma_1)(1 + \sigma_1) \\
& + \frac{2(1 - \sigma_1)}{E_1} b \left[ -\bar{\tau}_{rr_1} \left( \frac{5}{6} - \sigma_1 \right) + \bar{\tau}_{r\theta} \left( \frac{2}{3} - \sigma_1 \right) \right]
\end{aligned} \tag{90}$$

$$\begin{aligned}
& \frac{1 + \sigma_2}{E_2} \left[ (6 - 4\sigma_2) C_6 b^3 + 2C_5 b + 2 \frac{C_7}{b^3} - \frac{2(1 - 2\sigma_2)C_8}{b} \right] = \frac{2\tau_0 b}{E_1} \left[ (1 - 2\sigma_1)(1 + \sigma_1) \right] \\
& - \frac{(1 + \sigma_1)}{E_1} b \left[ -\bar{\tau}_{rr_1} \left( \frac{4}{3} - 2\sigma_1 \right) + \bar{\tau}_{r\theta_1} \left( \frac{5}{3} - 2\sigma_1 \right) \right]
\end{aligned} \tag{91}$$

## COMPUTATION AND NUMERICAL RESULTS

Equations (81), (82), (83), (85), (86), (87), (89), (90) and (91) form a system of nine equations for the nine unknowns. For compatibility with the nondimensional dynamic analysis presented previously, and to expedite computation, the following notation and assumptions are introduced:

$$\frac{1 - \sigma_1}{E_1} = \frac{2}{\mu_1}, \quad \frac{1 - \sigma_2}{E_2} = \frac{2}{\mu_2}, \quad \frac{\mu_2}{\mu_1} = \frac{1}{\nu}$$

$$a = 1, \quad \tau_0 = 1, \quad b = \frac{b}{a} = \eta \tag{92}$$



Equation (92) automatically correctly nondimensionalizes all the stress expressions. The nine equations then become

$$2C_2 + C_3 = 0$$

$$2C_5 + 6C_7 + 4C_8 = 0$$

$$2C_5 + 6C_6 - 6C_7 - 2C_8 = 0$$

$$2C_5 + \frac{6C_7}{\eta^4} + \frac{4C_8}{\eta^2} = -\bar{\tau}_{rr_1}$$

$$2C_2 + \frac{C_3}{\eta^2} = \bar{\tau}_{rr_0}$$

$$2C_5 + 6C_6\eta^2 - \frac{6C_7}{\eta^4} - \frac{2C_8}{\eta^2} = \bar{\tau}_{r\theta_1}$$

$$(1 - 2\sigma_2)(2C_2) - \frac{C_3}{\eta^2} = \frac{1}{\nu}(-1 - \bar{\tau}_{rr_0})$$

$$4\sigma_2 C_6\eta^2 + 2C_5 - \frac{2C_7}{\eta^4} - (1 - \sigma_2) \frac{4C_8}{\eta^2}$$

$$= \frac{1}{\nu} \left\{ 2(1 - 2\sigma_1) - 2 \left[ -\bar{\tau}_{rr_1} \left( \frac{5}{6} - \sigma_1 \right) + \bar{\tau}_{r\theta_1} \left( \frac{2}{3} - \sigma_1 \right) \right] \right\}$$

$$(6 - 4\sigma_2) C_6\eta^2 + 2C_5 + \frac{2C_7}{\eta^4} - \frac{2(1 - \sigma_2)C_8}{\eta^2}$$

$$= \frac{1}{\nu} \left\{ 2(1 - 2\sigma_1) + \left[ \bar{\tau}_{rr_1} \left( \frac{4}{3} - 2\sigma_1 \right) - \bar{\tau}_{r\theta_1} \left( \frac{5}{3} - 2\sigma_1 \right) \right] \right\} \quad (93)$$

Equation (91) is solved for  $C_2 \dots C_8$ ,  $\bar{\tau}_{rr_0}$ ,  $\bar{\tau}_{rr_1}$ , and  $\bar{\tau}_{r\theta_1}$ , with  $\eta = 1.05, 1.1, 1.2$  and two cases of material properties corresponding to the soft and stiff cylindrical lining of the dynamic analysis.

#### Soft Cylindrical Lining

$$\nu = 2.9, \sigma_1 = .25, \sigma_2 = .20$$

#### Stiff Cylindrical Lining

$$\nu = .31, \sigma_1 = .25, \sigma_2 = .30$$

The values of constants  $C_2 \dots C_8$ ,  $\bar{\tau}_{rr_0}$ ,  $\bar{\tau}_{rr_1}$ , and  $\bar{\tau}_{r\theta_1}$  for these two cases are tabulated in Table I.

Table I

Case I $\nu = 2.9 \quad \sigma_1 = .25 \quad \sigma_2 = .20$									
$\eta$	$C_2$	$C_3$	$C_5$	$C_6$	$C_7$	$C_8$	$\bar{\tau}_{rr_0}$	$\bar{\tau}_{rr_1}$	$\bar{\tau}_{r\theta_1}$
1.05	-.112	.224	.053	.002	.057	-.112	-.021	.019	.041
1.1	-.116	.232	.053	.003	.060	-.116	-.040	.033	.077
1.2	-.123	.246	.055	.004	.063	-.122	-.075	.047	.133
Case II $\nu = .31 \quad \sigma_1 = .25 \quad \sigma_2 = .30$									
$\eta$	$C_2$	$C_3$	$C_5$	$C_6$	$C_7$	$C_8$	$\bar{\tau}_{rr_0}$	$\bar{\tau}_{rr_1}$	$\bar{\tau}_{r\theta_1}$
1.05	-.1004	2.008	.596	-.052	.491	-1.035	-.187	.138	.298
1.1	-.903	1.806	.595	-.069	.458	-.984	-.313	.187	.442
1.2	-.775	1.551	.572	-.071	.429	-.930	-.474	.197	.578

Using the values of the constants tabulated in Table I, it is possible to evaluate the stresses and displacements at the boundaries of the stiff and soft cylinder. For purposes of this report,  $\tau_{\theta\theta(2)}^*$ ,  $\tau_{\theta\theta(1)}^*$ , and  $\tau_{rr(1)}^*$  are

$$r = a \quad r = b \quad r = b$$

calculated for each case. The (\*) notation is consistent with the nondimensional notation used in the dynamic analysis.

$$\tau_{\theta\theta(2)}^* = 2C_2 - C_3 + (2C_5 + 12C_6 + 6C_7) \cos 2\theta$$

$$r = a$$

$$\tau_{\theta\theta(1)}^* = \tau_{\theta\theta(1)}' + \tau_{\theta\theta(1)} = \frac{1}{1 - \sigma_1} \left[ -1 + (2 - 4\sigma_1) \cos 2\theta \right] - \bar{\tau}_{rr0}$$

$$r = b \quad r = b \quad r = b$$

$$+ (\bar{\tau}_{rr1} - 2\bar{\tau}_{r\theta1}) \cos 2\theta \quad (94)$$

$$\bar{\tau}_{rr(1)}^* = \bar{\tau}_{rr0} + \bar{\tau}_{rr1} \cos 2\theta$$

$$r = b$$

These values for  $\theta = \pi, \pi/2$  and  $\eta = 1.05, 1.1, 1.2$  are plotted on Figs. 3, 4, 5, 6, 8, 9, 10 and 11 at  $\alpha_1 a = 0$ ; this represents the infinite wave length or static solution.

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